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# 400VXS144-22 VIPER-X INTERIOR STUD

## **Geometric Properties**

4" x 1-7/16" flange, Viper-X Studs are manufactured from standard G40 hot-dipped galvanized steel. G60 and G90 coatings are available through special order, and may require up-charges and extended lead times.

### **Steel Thickness**

Member	Design Thickness (in)	Minimum Thickness (in)	<b>Yield</b> (ksi)	Web Sizes (in)	Coating <sup>4,5</sup>	Flange (in)	Return Lip (in)	-	<b>──</b> 1.4375" ──►
400VXS144-22	0.0235	0.0223	57	4	G40	1-7/16	3/8		
otes: 1. Uncoated s ad is the minimum acc G60 and G90 availab	ceptable thickne: Ie upon request.	ss. <b>3.</b> Knockout s Will require exte	size for 1-5/8″ ended lead tin	Stud is 3/4" x 2".					
Color Code (p STM & Code ASTM A653/A IAPMO ER-052 IBC: 2012, 201 CBC: 2013, 201	<b>Standarc</b> 653M, A924, 4 5, 2018, 2021	<b>is:</b> /A924M, A10		C645 & C754, E	119			"W" — ►	"Т"
AISI: S100, S2	20							Web Depth	Steel Thickness
EED v4 for B MR Prerequisit MR Credit: Cor	te: Constructi	on and Demo	lition Was	te Manageme	nt Planning.				

- MR Credit: Construction and Demolition Waste Management.
- MR Credit: Building Product Disclosure and Optimization Sourcing of Raw Materials, Option 2.
- MR Credit: Building Product Disclosure and Optimization Environmental Product Declarations, Options 1 & 2.
- MR Credit: Building Product Disclosure and Optimization Material Ingredients, Option 1.
- MR Credit: Building Life-Cycle Impact Reduction, Option 4.

#### **CEMCO** cold-formed steel framing products contain 30% to 37% recycled steel.

■ Total Recycled Content: 36.9% ■ Post-Consumer: 19.8% ■ Pre-Consumer: 14.4%

CSI Division: 09.22.16 - Non-Structural Metal Framing

### 400VXS144-22 Viper-X Section and Structural Properties

				Gross Properties						Effective Properties				Torsional Properties					
Yield	Web	Design																	Critical
		Design	Woight	Aree	ly.	Rx	by.	Rv	lvo	Sxe	Mail	Ma-d	Vaq	J (x 10 <sup>-6</sup> )	Cw	Хо	Ro		Unbraced
		Thickness <sup>3</sup> , t			IX		IV		Ixe		Ma-I			1					Length,
(ksi)	(in)	(in)	(lb/ft)	(in <sup>2</sup> )	(in <sup>4</sup> )	(in)	(in <sup>4</sup> )	(in)	(in <sup>4</sup> )	(in <sup>3</sup> )	(k-in)	(k-in)	(k)	(in <sup>4</sup> )	(in <sup>6</sup> )	(in-k)	(in-k)	IS	Lu (in)
57	4.000	0.0235	0.597	0.176	0.435	1.574	0.046	0.512	0.423	0.159	5.355	4.611	0.686	32.341	0.153	-0.970	1.918	0.744	26.76

Notes: 1. Web height to thickness ratio (h/t) exceeds 200. Web stiffeners required at all support points and concentrated loads.
2. Members having a web height to thickness ratio (h/t) value exceeding 260 will not have effective properties listed, only gross properties will be listed.
3. Web height value (h) used for h/t

calculation is the flat width of the web. For (S) members, this is the out to out member size, minus twice the thickness, minus twice the inside bend radius. **4.** Members having a flange width to thickness ratio (b/t) value exceeding 60 must be considered for use with the limitations described in AISI S100 & S220 section B1. **5.** Flange

width value (b) used for b/t calculation is the flat width of the flange. For (S) members, this is the out to out member size, minus twice the thickness, minus twice the inside bend radius.

# Non-Composite Limiting Heights – Braced at 48" O.C.

Depth		Yield	Design Thickness (in)	Spacing O.C. (in)		5 PSF			7.5 PSF		10 PSF		
(in)	Member	(ksi)			L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
	400VXS144-22	57	0.0235	12	22' 4"	17' 9"	15' 6"	19' 6"	15' 6"	13' 6"	17' 2" f	14' 1"	12' 4"
4	400VXS144-22	57	0.0235	16	20' 5"	16' 3"	14' 2"	17' 3" f	14' 2"	12' 5"	14' 11" f	12' 11"	11' 3"
	400VXS144-22	57	0.0235	24	17' 2" f	14' 1"	12' 4"	14' 0" f	12' 3"	10' 9"	12' 2" f	11' 2"	9' 9"

Notes: 1. Web height to thickness ratio (h/t) exceeds 200. Web stiffeners required at all support points and concentrated loads.
2. Lateral loads of 5 psf, 7.5 psf, and 10 psf have NOT been reduced for strength or deflection checks. Full lateral load is applied.
3. Limiting heights are in accordance with AISI S100 & S220 using

all steel non-composite design. **4.** Limiting heights are established by considering flexure (f), web crippling (w) and deflection. **5.** Allowable moment is the lesser of Mal and Mad. Stud distortiona buckling based on an assumed KD = 0. **6.** For bending, studs are assumed to be adequately braced to develop full allowable moment.

7. Web crippling check is based on AISI S100 & S220 section C3.4.2 Condition 1: End One-Flange Loading with 1" end bearing.



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# 400VXS144-22 VIPER-X INTERIOR STUD

# Non-Composite Limiting Heights – Fully Braced

Depth		Design	Min (in)	Yield (ksi)	Spacing (o.c.)		5 PSF			7.5 PSF		10 PSF		
(in)	Member	(in)				L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
	400VXS144-22	0.0235	0.0223	57	12	22' 4"	17' 8"	15' 6"	19' 6"	15' 6"	13' 6"	17' 6" f	14' 1"	12' 3"
4	400VXS144-22	0.0235	0.0223	57	16	20' 5"	16'3"	14' 2"	17' 6" f	14' 2"	12' 5"	15' 2" f	12' 11"	11' 3"
	400VXS144-22	0.0235	0.0223	57	24	17' 6" f	14' 1"	12' 3"	14' 3" f	12' 3"	10' 9"	12' 4" f	11' 2"	9' 9"

Notes: 1. Web height to thickness ratio (h/t) exceeds 200. Web stiffeners required at all support points and concentrated loads. 2. Lateral loads of 5 psf, 7.5 psf, and 10 psf have NOT been reduced for strength or deflection checks. Full lateral load is applied. 3. Limiting heights are in accordance with AISI S100 & S220 using all steel non-composite design. 4. Limiting heights are established by

considering flexure (f), web crippling (w) and deflection. 5. Allowable moment is the lesser of Mal and Mad. Stud distortional buckling based on an assumed  $K\Phi=0$ . 6. For bending, studs are assumed to be adequately braced to develop full allowable moment. 7. Studs are fully braced when unbraced length is less than Lu. See section properties table for Lu values. 8. Web crippling check is based on

AISI S100 & S220 section C3.4.2 Condition 1: End One-Flange Loading with 1" end bearing.

## Allowable Composite Heights for Non-Load Bearing Walls

Depth		Design (in)	Min (in)	Yield (ksi)	Spacing (o.c.)	5 PSF			7.5 PSF			10 PSF		
(in)	Member					L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
	400VXS144-22	0.0235	0.0223	57	12	24'-9"	19'-8"	17'-2"	21'-8"	19'-0"	15'-0"	19'-8"	15'-7"	13'-8"
4	400VXS144-22	0.0235	0.0223	57	16	22'-6"	17'-11"	15'-8"	19'-8"	15'-8"	13'-8"	17'-9"	14'-2"	12'-5"
	400VXS144-22	0.0235	0.0223	57	24	19'-8"	15'-7"	13'-8"	16'-11"	13'-8"	11'-11"	15'-0"	12'-5"	10'-8"

Notes: 1. Viper composite limiting heights are based on testing in accordance with ICC-ES acceptance criteria AC86. 2. Limiting heights are established by considering flexure, shear, web crippling, and deflection. 3. Mechanical fastening of gypsum panel to the stud and track is required, except when installing a minimum 30 mil slotted

track with 2-1/2" legs in lieu of standard track. **4.** Viper-X composite limiting heights based on a single layer of 5/8" type X gypsum board applied vertically to both sides of the wall over full height. 5/8" Type X wallboard from the following manufacturers are acceptable: USG, National, Georgia-Pacific, PABCO, CertainTeed, & American. **5.** For deflection track usage contact Technical Services. **6.** For GWB installed horizontally, see table for "Non-Composite Limiting Heights-Fully Braced (see above).

## Screw Allowable Loads (lbs.)

	Design Thickness	Min. Thickness	Fy Yield	Fu Tensile	#6 SCREW (0.138" Dia; 0.25" Head)		#8 SCREW (0.164" Dia; 0.3125" Head)		#10 SCREW (0.190" Dia; 0.340" Head)		#12 SCREW (0.216" Dia; 0.340" Head)	
Member	(in)	(in)	(ksi)	(ksi)	Shear	Tension	Shear	Tension	Shear	Tension	Shear	Tension
400VXS144-22	0.0235	0.0223	57	65	174 <sup>1</sup>	60	184 <sup>1</sup>	71	236 <sup>1</sup>	82	152	93

Notes: 1. Shear values are tested per AISI S100, S220 & S905 procedure. 2. Capacities are based on section E4 of the AISI S100 & S220 Specification. 3. Capacities are based on Allowable Strength Design (ASD). 4. Screw pull-out capacities are based on listed head diameter. 5. Two sheets of equal thickness and tensile strength are assumed in tabulated values. 6. When materials of different steel thickness and tensile strength are connected, use the lowest value for shear capacity (tilting and bearing), for pull-out capacity use sheet closest to screw tip and for pull-over capacity use sheet closest to screw head. **7.** Where multiple fasteners are used, screws are assumed to have a center-to-center spacing of at least 3 times the nominal diameter. **8.** Screws are assumed to have a center-of-screw to edge-of-steel dimension of at least 1.5 times the nominal diameter of the screw. **9.** When screws are subjected to combination of shear and tension forces, interaction equation of AISI S100 & S220 Specification section E4.5 shall be used.

Technical Services Technical Services: 800 416 2278





