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Structural Engineering/Design

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Technical Services 13191 Crossroads Pkwy N., Ste 325 City of Industry, CA 91746

City of Industry, CA 91746 Phone: 800.416.2278 Fax: 626.249.5004

400VS125-15 (25EQ) VIPERSTUD

Geometric Properties

4" x 1-1/4" flange, 15 mil ViperStuds are manufactured from standard G40 hot-dipped galvanized steel. G60 and G90 coatings are available through special order, and may require up-charges and extended lead times.

Steel Thickness

Model No.	Design Thickness (in) (in)		Yield (ksi)	"W" Web Sizes (in)	Coating ^{4,5}	Flange (in)	"L" Return Lip (in)	
400VS125-15 (25EQ)	0.0155	0.0147	50	4	G40	1-1/4	1/4	

Notes: 1. Uncoated steel thickness. Thickness is for carbon sheet steel. 2. Minimum thickness represents 95% of the design thickness and is the minimum acceptable thickness. 3. Knockout size for 4" Stud is 1-1/2" x 2-1/2". 4. Per ASTM C645 & A1003, Table 1.
 5. G60 and G90 available upon request. Will require extended lead time and upcharge.

Color Code (painted on ends): 15 mil: None with Dark Grey band on pallet

ASTM & Code Standards:

- ASTM A653/A653M, A924/A924M, A1003/1003, C645 & C754
- ICC-ES & SFIA Code Compliance Certification Program
- ICC ESR-2620
- CBC: 2016, 2019, 2022
 AISI: S100, S220
- IBC: 2015, 2018, 2021 AISI: S100, S220 LEED v4 for Building and Design Construction
- MR Prerequisite: Construction and Demolition Waste Management Planning.
- MR Credit: Construction and Demolition Waste Management.
- MR Credit: Building Product Disclosure and Optimization Sourcing of Raw Materials, Option 2.

Gross Properties

rx

(in)

0.108 0.250 1.520 0.020 0.429

■ MR Credit: Building Product Disclosure and Optimization – Environmental Product Declarations, Options 1 & 2.

ly

(in⁴)

rv

(in)

- MR Credit: Building Product Disclosure and Optimization Material Ingredients, Option 1.
- MR Credit: Building Life-Cycle Impact Reduction, Option 4.

Yield

(ksi)

50

Design

(in)

0.0155 0.0147

Min

(in)

Weight

(lb/ft)

0.37

Notes: 1. Nominal Moments for Viper25 are based on testing.

Allowable moment (Ma) is calculated with safety factor of 1.81

in accordance with chapter F of AISI S100-16/S2-20 specification.

2. Nominal moment for Viper20, Viper 30mil, Viper 33mil and

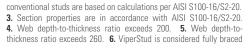
400VS125-15⁴ (25EQ)ViperStud Properties

Area

(in²)

lx

(in4)



Sx

(in³)

0.061

Allowable

Moment

Ma (in-k)

1.69

Effective Properties

lxd

(in⁴)

0.255

when the unbraced length is less than listed Lu. 7. $K\Phi$ assumed to be zero for distortional buckling moments.

Nominal Moment

for Conventional

Studs³

Mn (in-k)

2.74 (18 mil)

Critical

Unbraced

Length⁷

Lu (in)

24.4

Moment

Distortional

Buckling Nominal

Moment² Viper

Mnd (in-k)

3.06

Local Buckling

Nominal Moment²

Viper

Mnl (in-k)

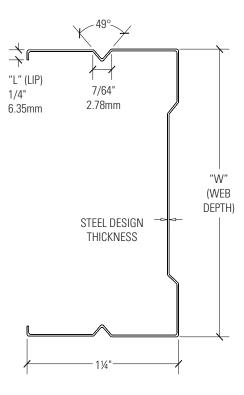
3.06

Non-Composite Limiting Heights – Braced at 48" O.C.

Depth		Member	Design (in)	Min (in)	Yield (ksi)	Spacing (o.c.)	5 PSF			7.5 PSF			10 PSF		
(in)	Gauge	Designation					L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
		400VS125-156 (25EQ)	0.0155	0.0147	50	12	13'-10" f	13'-10"f	13'-1"	11'-4"f	11'-4" f	11'-4" f	9'-10" f	9'-10" f	9'-10" f
4	25EQ	400VS125-156 (25EQ)	0.0155	0.0147	50	16	12'-0" f	12'-0"f	11'-11"	9'-10"f	9'-10" f	9'-10" f	7'-5" w	7'-5" w	7'-5" w
		400VS125-156 (25EQ)	0.0155	0.0147	50	24	9'-10" f	9'-10"f	9'-10"f	6'-6"w	6'-6" w	6'-6" w			

Notes: 1. Limiting heights are in accordance with AISI S100-16/ S2-20 using all steel non-composite design. 2. Limiting heights are established by considering flexure, shear, web crippling and deflection. 3. Lateral-Torsional buckling moments are based on section F of AISI S100-16/S2-20, with max discrete bracing of 48" o.c. 4. For web crippling, when h/ts 200, the web crippling values are computed based on section G6 of AISI S100-16/S2-20, when h/t>200, the web crippling values are based on testing with a bearing length of 1". 5. No web stiffeners are required for studs with h/t < 200, web crippling and shear values have been confirmed by testing. 6. The factory punchouts are in accordance with AlSI standards. The distance from the center of the last punchout to the end of the stud is 12". 7. Use non-composite tables when 1/2 inch gypsum board, horizontal board, RC channel, furring channel, or sound clips are used. 8. Review fire rated assemblies for additional requirements.

"f" - flexure controls; "s" - shear controls; "w" - web crippling controls. No letter next to the number means deflection controls.





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400VS125-15 (25EQ) VIPERSTUD

Non-Composite Limiting Heights – Fully Braced

Depth		Member	Design (in)	Min (in)	Yield (ksi)	Spacing (o.c.)	5 PSF			7.5 PSF			10 PSF		
(in)	Gauge	Designation					L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
		400VS125-156 (25EQ)	0.0155	0.0147	50	12	15'-0" f	15'-0"	13'-1"	12'-4" f	12'-4" f	11'-5"	10'-7" f	10'-7" f	10'-5"
4	25EQ	400VS125-156 (25EQ)	0.0155	0.0147	50	16	13'-0" f	13'-0" f	11'-11"	10'-7" f	10'-7" f	10'-5"	9'-2" f	9'-2" f	9'-2"f
		400VS125-156 (25EQ)	0.0155	0.0147	50	24	10'-7" f	10'-7" f	10'-5"	8'-6" w	8'-6" w	8'-6" w	6'-5" w	6'-5" w	6'-5"w

Notes: 1. Limiting heights are in accordance with AISI S100-16/S2-20 using all steel non-composite design. 2. Limiting heights are established by considering flexure, shear, web crippling, and deflection. 3. For bending, studs are assumed to be adequately braced to develop full allowable moment. Studs are considered fully braced when unbraced length is less the Lu. See section properties table on page 5 for Lu values. 4. For web crippling, when h/t \leq 200, the web crippling values are computed based on section G6 of AISI S100-16/S2-20, when h/t

>200, the web crippling values are based on testing with a bearing length of 1". **5.** No web stiffeners are required for studs with h/t < 200, web crippling and shear values have been confirmed by testing. **6.** The factory punchouts are in accordance with AISI standards. The distance from the center of the last punchout to the end of the stud is 12". **7.** Use non-composite tables when 1/2 inch gypsum board, horizontal board, RC channel, furring channel, or sound clips are used. **8.** Review fire rated assemblies for additional requirements.

"f" - flexure controls; "s" - shear controls; "w" - web crippling controls. No letter next to the number means deflection controls.

Allowable Composite Heights for Non-Load Bearing Walls

Depth		Member	Design (in)	Min (in)	Yield (ksi)	Spacing (o.c.)	5 PSF			7.5 PSF			10 PSF		
(in)	Gauge	Designation					L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
	25EQ	400VS125-15 (25EQ)	0.0155	0.0147	50	12	22'-1″	18'-3″	16'-3″	19'-3″	15'-11″	14'-2"	16'-8"	14'-6″	12'-11"
4		400VS125-15 (25EQ)	0.0155	0.0147	50	16	20'-0"	16'-7″	14'-9″	16'-8″	14'-6″	12'-11"	14'-5″	13'-2″	11'-9"
		400VS125-15 (25EQ)	0.0155	0.0147	50	24	16'-8″	14'-6″	12'-11″	13'-7″	12'-8″	11'-3"	11'-9″	11'-6″	10'-1"

Notes: 1. Sheathing, as specified in Section 3.2.2, must be attached to both faces of the wall for the full height of the wall with the long dimension parallel to the studs. 2. Sheathing must be fastened to the studs with fasteners as specified in Section 3.2.3 and installed per Section 4.2.1. 3. Placement of joints in the gypsum sheathing must be in accordance with Sections 4.6.3 and 4.6.4 of GA-216 or Section 7.5 of ASTM C840. 4. The bottom and top tracks are xxxVT125 (solid flange

track). A minimum 30 mil slotted flange track (xxxCST250 or xxxSLT250) may be used for the top track. **5.** End-bearing must be a minimum of 1 inch for xxxVT125 (solid flange track) and 1-5/8 inches for xxxCST250 or xxxSLT250 (slotted flange track). **6.** Notes 1, 2, & 3 are referenced in ICC ESR 2620 page 5. **7.** For any other top tracks not listed in note 4, please contact technical services for assistance. **8.** For GWB installed horizontally, see table for "Non-Composite Limiting Heights- Fully Braced" (see above)

Allowable Ceiling Spans

L/240			4 PSF Late	ral Support	of Compress	ion Flange	6 PSF Lateral Support of Compression Flange						
Member	Fy	Unsupp	oorted Joist ((in.) O.C.	Spacing	Midspan Joist Spacing (in.) O.C.			Unsupported Joist Spacing (in.) O.C.			Midspan Joist Spacing (in.) O.C.		
	ksi	12	16	24	12	16	24	12	16	24	12	16	24
400VS125-15 (25EQ)	50	9'-5"f	8'-9"f	7'-10"f	12'-5"f	11'-4"f	10'-0"f	8'-6"f	7'-10"f	7′-1″f	11'-0"f	10'-0"f	8'-9"f

L/360			4 PSF Late	ral Support	of Compress	ion Flange	6 PSF Lateral Support of Compression Flange							
Member	Fy	Unsupp	orted Joist S (in.) O.C.	Spacing	Midsp	oan Joist Sp (in.) O.C.	an Joist Spacing Unsupported Joist S (in.) O.C. (in.) O.C.			Spacing	Mids	pan Joist Sp (in.) O.C.	n Joist Spacing in.) O.C.	
	ksi	12	16	24	12	16	24	12	16	24	12	16	24	
400VS125-15 (25EQ)	50	9'-5"f	8'-9"f	7'-10"f	12'-5"f	11'-4"f	10'-0"f	8'-6"f	7'-10"f	7′-1″f	11'-0"f	10'-0"f	8'-9"f	

Notes: 1. Ceiling Spans are in accordance with AISI S100-16/S2-20 using all steel non-composite design. 2. Ceiling Spans are established by considering flexure, shear, web crippling and deflection. 3. For web crippling, when $h/t \le 200$, the web crippling values are computed based on G6 of AISI S100-16/S2-20. When h/t > 200, the web crippling values are based on testing with a bearing length of 1". 4. No web stiffeners are required for studs with h/t < 200, web crippling

and shear values have been confirmed by testing. **5.** All values are for simple spans, with compression flange either unbraced or braced at midspan. **6.** Ceiling spans are based on total load of assembly, not including storage or live load for accessible ceilings. **7.** The factory punchouts are in accordance with AISI standards. The distance from the center of last punchout to the end of the stud is 12".

"f" - flexure controls; "s" - shear controls; "w" - web crippling controls. No letter next to the number means deflection controls.

CEMCO cold-formed steel framing products contain 30% to 37% recycled steel.

■ Total Recycled Content: 36.9% ■ Post-Consumer: 19.8%

9.8% Pre-Consumer: 14.4%

CSI Division:

09.22.16 – Non-Structural Metal Framing

Check the updated list of Certified Production Facilities at Intertek's website at http://www.intertek.com/building/sfia







This technical information reflects the most current information available and supersedes any and all previous publications effective February 20, 2024. 02/20/2024 AT

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